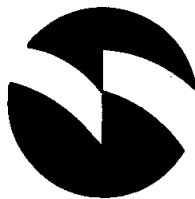
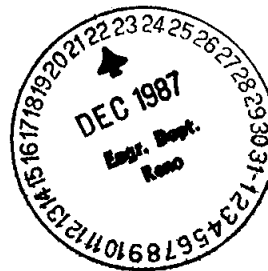




HYDROLOGY REPORT

FOR

WASHOE MEDICAL CENTER

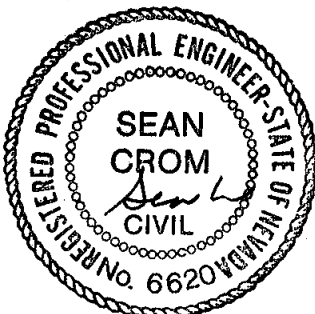


PREPARED BY:

OMNI-MEANS, LTD.
6121 LAKESIDE DRIVE, SUITE 100
RENO, NEVADA 89511

TELEPHONE: (702) 825-1223

DECEMBER 22, 1987



12-23-87

INTRODUCTION

The Washoe Medical Center is located in the City of Reno between Mill Street and East Second Street. See Vicinity map on plate 1.

According to F E M A maps dated January 5, 1984, no portion of the site is within the 100 year flood plain.

The Washoe Medical Center consists of the existing hospital and other smaller structures detached from it. Construction of two additional buildings, the Ambulatory Care Center and the N.H.S. Office Building along with additional parking lots are nearing completion. Plans for a proposed 2 story parking structure are under way and construction is expected to start in January 1988.

There is a capacity problem with the existing offsite storm drain system which handles storm drainage from the Washoe Medical Center, the Ronald McDonald House, the Washoe County Administration Building, and the Senior Citizens Complex. In an effort to reduce the existing offsite drainage problems, Washoe Medical Center will revise its current drainage system which was approved and constructed in conjunction with the 2 new buildings. Onsite percolation pits will be proposed to reduce storm water runoff leaving the site. With this revision, storm water runoff leaving the site will be much less than it was prior to overall construction.

EXISTING CONDITIONS

(See plate 2)

The existing area consists of three paved parking lots, Pringle Way, and several buildings in the north east portion of the site are surrounded by either concrete, grass or dirt. The landscaped or non-impervious area amounts to approximately 26% of the total area.

Existing drainage in the area affected by the development travels to two different offsite locations, East Second Street and Mill Street. East Second Street collects overland storm drainage flow from the Northeast portion of the site, Area (E)A. The majority of the onsite storm water runoff, Area (E)B, is taken to a 24" storm drain line in Mill Street. Storm water drainage leaves the site in a 21" storm drain line at Pringle Way and at a V-ditch at the East property line. The 21" storm drain line and the V-ditch converge at offsite manhole EM2. From there the 21" line continues to a manhole in the Senior Citizens Complex at which point it is reduced to a 15" line. The 15" line then connects to the 24" storm drain line in Mill Street.

CURRENT DEVELOPMENT UNDER CONSTRUCTION

(See plate 3)

The current development under construction consists of the addition of the N.H.S. Office Building, the Ambulatory Care Center, and associated parking lots. The storm water runoff will be directed to the same locations as the existing condition, but the methods of channelization change. The Northeast portion of the site, Area A, will channelize sheet flow to a catch basin with a connecting pipe to the existing storm drain line in East Second Street. The

main (eastern) parking lot, and the N.H.S. Office Building, Area B, will channelize sheet flow to a new 18" storm drain line which will pipe the runoff directly to existing manhole EM2. The Ambulatory Care Center and Pringle Way, Area C, will channelize sheet flow and pipe the runoff to the existing 21" storm drain line which goes to the Mill Street storm drain line.

PROPOSED PARKING STRUCTURE
(See plate 3)

The existing 21" storm drain line will be rerouted through the parking structure and collect the parking structure's storm water runoff. The rerouted 21" storm drain line will connect back into existing manhole EM1 at Pringle Way. This manhole will be modified so that storm water coming into the manhole will be piped to a percolation pit. The pit will be designed to handle the 10 year, 6 hour storm for the parking structure. Any excess storm water that the pit can not handle will overflow back into the existing 21" storm drain line and continue to the Mill Street storm drain line.

REVISIONS TO CURRENT DEVELOPMENT
(Refer to plate 3)

The storm water runoff in Area B will be discharged onsite through a percolation pit and will not be piped offsite to the storm drain line in Mill Street. The 18" Storm drain line recently installed will be plugged at manhole P1 and abandoned between manhole P1 and the existing offsite storm drain manhole EM2. The onsite 18" storm drain line will connect into the percolation pit.

RESULTS

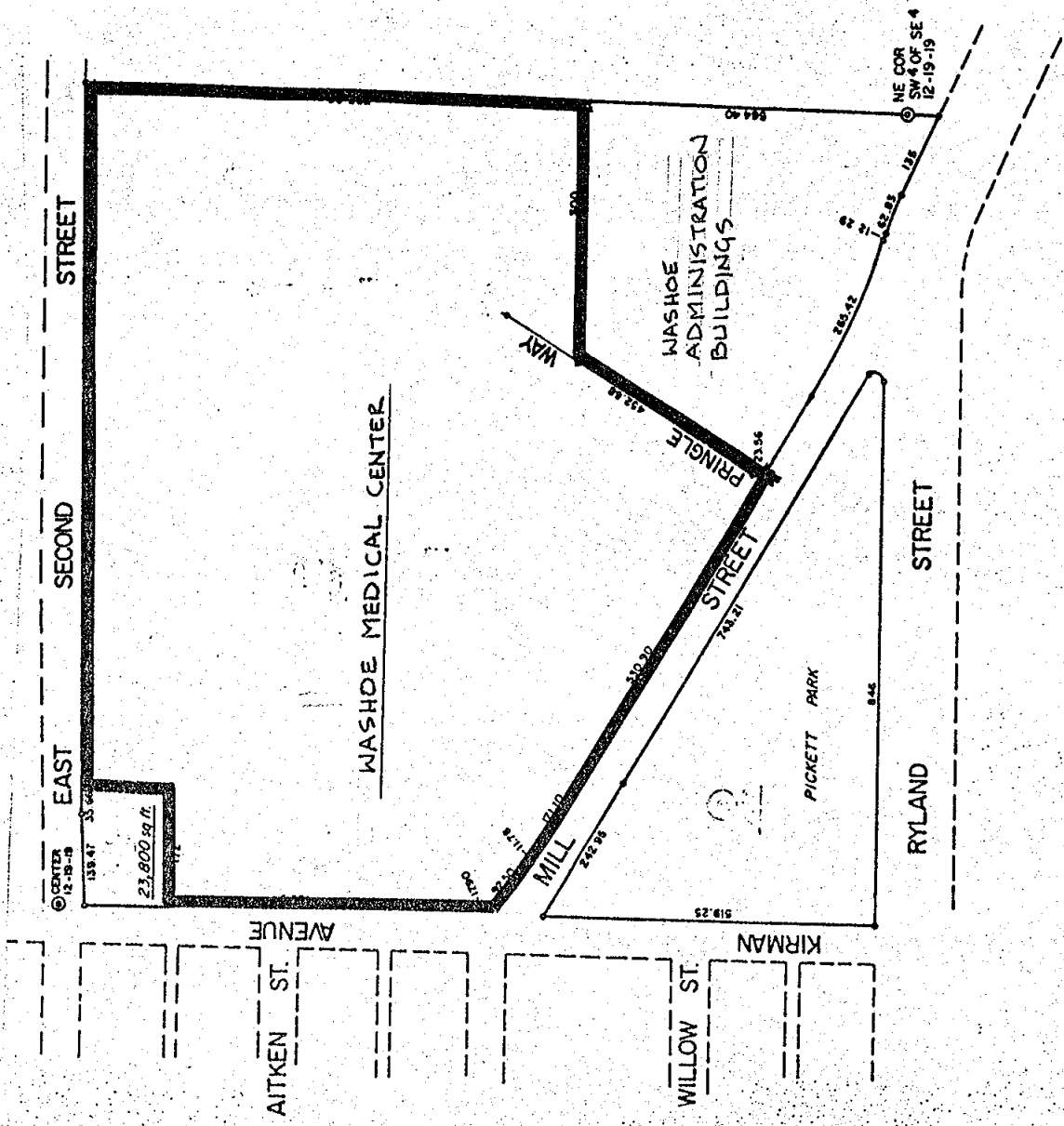
Tables 1 AND 2 summarize the flows for the existing site and the proposed development. When comparing existing Area (E)A to proposed Area A, (see plates 2 & 3), the flow to East Second Street increases slightly (0.48 cfs for the 10 year storm).

The other result which must be addressed is the comparison between existing Area (E)B and proposed Areas B, C, and D. The 10 year storm flow exiting the site from existing Area (E)B to the Mill Street storm drain line is 12.26 cfs. If proposed Areas B, C, and D are combined together, the resulting 10 year storm flow is 13.78 cfs. To reduce this flow, percolation pits to infiltrate storm water back into the ground onsite will be constructed for zones B, and D. The pits will be designed to accommodate a 10 year , 6 hour storm. If this is exceeded, overflow will be directed to the Mill Street storm drain line. This will reduce the 10 year flow exiting the site into the Mill Street storm drain line to 3.33 cfs, a decrease of 76% under existing storm runoff flows.

Calculations for storm water runoff and percolation pit designs for zones B and D are given in appendix B. In order to determine the percolation rate used for design, test pits were constructed onsite by two separate geotechnical firms. The results of both firms were very similar (see appendix C) yielding a percolation rate of 1 gallon per minute per square foot of surface area. The surface area used in our design consists of the bottom area and one third of the side wall area of the pits.

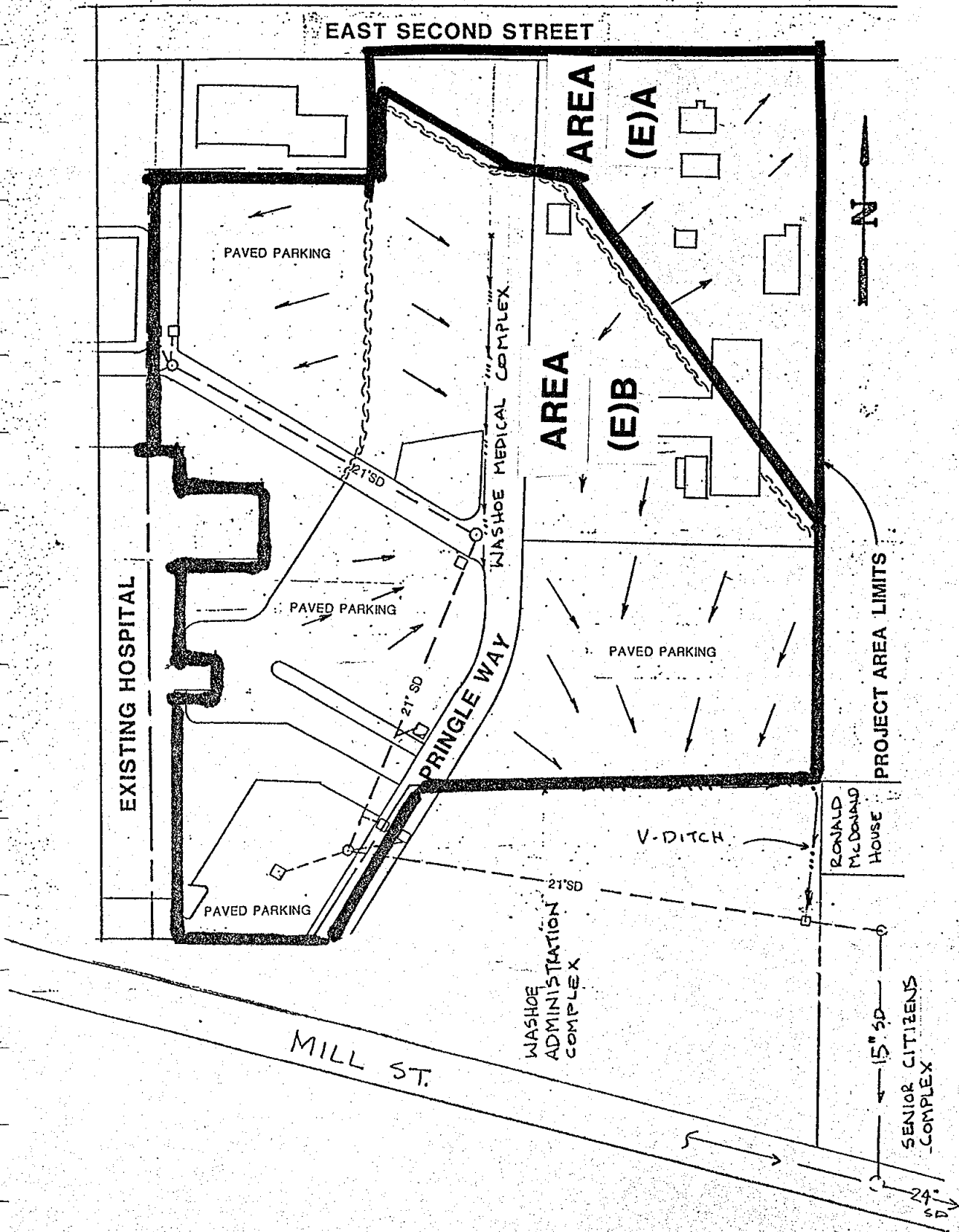
CONCLUSION

The onsite subsurface conditions of the site are very favorable for the infiltration of storm water runoff back into the ground. By taking advantage of these conditions storm water runoff leaving the site to the Mill Street storm line will be decreased more than half (76 percent) then that of existing conditions prior to overall construction. This reduction in flow should help minimize the existing offsite problem of the storm drain system entering into the Mill Street storm drain line.



VICINITY MAP

EXISTING DRAINAGE CONDITIONS



EAST 2ND ST.

PHASE I LIMITS OF CONSTRUCTION

EXISTING GARAGE

ONLY

LANDSCAPE

43 SPACES @ 9'

38 SPACES @ 9'

38 SPACES @ 9'

34 SPACES @ 9'

34 SPACES @ 9'

34 SPACES @ 9'

34 SPACES @ 9'

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34 SPACES @ 9'

34 SPACES @ 9'

12" SD

15" SD

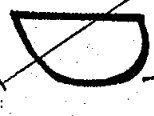
12" SD

2 1/2" SD

12" SD

B

16" SD



Existing M.H. EMI

N.H.S. OFFICE BUILDING
REFER TO OTHER DRAWINGS FOR BUILDING CONSTRUCTION

AMBULATORY CARE CENTER
REFER TO OTHER DRAWINGS FOR BUILDING CONSTRUCTION

WASHOE MEDICAL CENTER

PHASE

TABLE 1

STORM WATER RUNOFF FLOWS IN THE EXISTING AREA
EFFECTED BY THE PROPOSED DEVELOPMENT

(RATIONAL METHOD)

LOCATION*	DISCHARGE POINT	5-YEAR (CFS)	10-YEAR (CFS)	100-YEAR (CFS)
(E) A	EAST SECOND STREET STORM DRAIN	1.74	3.01	4.74
(E) B	MILL STREET	7.10	12.26	19.36

*REFER TO PLATE 2

TABLE 2

STORM WATER RUNOFF FLOWS AFTER PROPOSED DEVELOPMENT
(RATIONAL METHOD)

LOCATION* OF CONTRIBUTION AREA	DISCHARGE POINT	5-YEAR (CFS)	10-YEAR (CFS)	100-YEAR (CFS)
A	EAST SECOND STREET STORM DRAIN	2.02	3.49	5.51
B	ONSITE PERCOLATION PIT	4.46	7.20	11.37
C	MILL STREET STORM DRAIN	1.93	3.33	5.26
D	ONSITE PERCOLATION PIT	1.88	3.25	5.12

*REFER TO PLATE 3

APPENDIX A

STORM WATER RUNOFF CALCULATIONS

EXISTING AND PROPOSED

EXISTING AREA TO EAST SECOND ST.

(E) A

TOTAL AREA — 106,135[#] = 2.44 Ac.

LANDSCAPING AREA — 40,985[#] = 0.94 Ac.

IMPERVIOUS AREA — 65,340[#] = 1.50 Ac.

$$C_L = 0.25$$

$$C_I = 0.90$$

$$C_{AVG} = 0.65$$

$$Q_5 = C I A$$

$$Q_5 = 0.65(1.1) 2.44$$

$$Q_5 = 1.74 \text{ CFS}$$

$$Q_{10} = 3.01 \text{ CFS}$$

$$Q_{100} = 4.74 \text{ CFS}$$

EXISTING AREA TO 21" SD
 WHICH IS INFLUENCED BY THE
 PROPOSED DEVELOPEMENT (E) B

RATIONAL METHOD:

TOTAL AREA -	369,817 [#]	=	8.49 ACRES
LANDSCAPED " -	81,373 [#]	=	1.87 "
IMPERVIOUS " -	288,444 [#]	=	6.62 "

$C_L = 0.25$

$C_I = 0.90$

$$C_{AVG} = \frac{0.25(1.87) + 0.90(6.62)}{8.49}$$

$$= 0.76$$

$Q = C i \Delta$

$C = 0.76$

$t_c = 15 \pm$

$i_s = 1.1$

$Q_s = 0.76(1.1)(8.49)$
 $= 7.10 \text{ CFS}$

$Q_{10} = 12.26 \text{ CFS}$

$Q_{100} = 19.36 \text{ CFS}$

AREA "A" (PROPOSED)

(N. E. PARKING LOT)

TOTAL AREA	-	97,562 ^{sq}	= 2.24 Ac.
LANDSCAPED AREA	-	12,411 ^{sq}	= 0.28 "
IMPERVIOUS "	-	85,378 ^{sq}	= 1.96 "

$C_L = 0.25$

$C_I = 0.90$

$C_{AVG} = 0.82$

$Q_5 = C I A$

$Q_5 = 0.82 (1.1) (2.24)$

$Q_5 = 2.02 \text{ CFS}$

$Q_{10} = 3.49 \text{ CFS}$

$Q_{100} = 5.51 \text{ CFS}$

PROPOSED AREA "B" TO PERC. PIT

MAIN PARKING LOT (AREA) & N.H.S
BUILDING

TOTAL AREA - 194,141 sq ft = 4.46 ACRES
 LANDSCAPED " - 14,209 sq ft = 0.33 "
 IMPERVIOUS " - 179,932 sq ft = 4.13 "

$$C_L = 0.25$$

$$C_I = 0.90$$

$$C_{AVG} = \frac{4.13(0.90) + 0.33(0.25)}{4.46}$$

$$= 0.85$$

$$Q = C i A$$

$$C = 0.85$$

$$t_c = 15 \text{ min}$$

$$i = 1.1$$

$$A = 4.46$$

$$Q_5 = 0.85 (1.1) (4.46)$$

$$= 4.17 \text{ CFS}$$

$$Q_{10} = 7.20 \text{ CFS}$$

$$Q_{100} = 11.37 \text{ CFS}$$

AREA "C" PROPOSED

(AMBULATORY CARE CENTER & PRINGLE WAY)

TOTAL AREA - 93,744[#] = 2.19 Acres

LANDSCAPING - 15,447[#] = 0.35 "

IMPERVIOUS - 78,297[#] = 1.84 "

$C_L = 0.25$

$C_I = 0.90$

$C_{AVG} = 0.80$

$Q_5 = C_i A$
 $= 0.80(1.1) 2.19$
 $= 1.93 \text{ cfs}$

$Q_{10} = 3.33 \text{ cfs}$

$Q_{100} = 5.26 \text{ cfs}$

AREA "D" (PROPOSED)

(PROPOSED PARKING GARAGE)

TOTAL AREA -	85,275 [#]	= 1.96 Acres
LANDSCAPED " -	4,264 [#]	= 0.10 "
IMPERVIOUS -	81,022 [#]	= 1.86 "

$C_L = 0.25$

$C_I = 0.90$

$C_{AVG} = 0.87$

$Q_5 = C_i A$
 $= 0.87 (1.1) (1.96)$
 $= 1.88 \text{ CFS}$

$Q_{10} = 3.25 \text{ CFS}$
 $Q_{100} = 5.12 \text{ CFS}$

APPENDIX B

PERCOLATION PIT CALCULATIONS

Percolation PIT Calculations (AREA "B")

RAINFALL RUNOFF

$$\text{Total Area} = 194,141 \text{ ft}^2$$

$$\text{Landscaped Area} = 14,209 \text{ ft}^2$$

$$\text{Impervious Area} = 179,932 \text{ ft}^2$$

$$\text{Avg. Runoff coef.} = 0.85$$

Total rainfall precipitation for 10 yr 6hr storm

$$P = 1.2" \text{ (ref 1)*}$$

$$\text{Total Runoff Volume} = \frac{1.2}{12} \times 194,141 \times 0.85$$

$$V_R = 16,502 \text{ C.F.} = 123,435 \text{ gallons}$$

Perc. Pit Design

Storage area: USE $\frac{1}{3}$ Void ratio of the pit

$$\text{Percolation rate} = 1.0 \text{ gal/min/SF}$$

(per field test, see Appendix C)

Assume bottom surface area and $\frac{1}{3}$ side wall area to determine pit size.

$$\text{Try } 30 \times 15 \times 5' \\ 20 \times 20 \times 6' \text{ DP PIT}$$

$$\text{Actual STORAGE VOL. (SA)} = \frac{20 \times 20 \times 6}{3} = 800 \text{ C.F.} = 5984 \text{ gal}$$

Rate of percolation into soil:

$$P_s = 20 \times 20 + \frac{1}{3} \times 6(4 \times 20) = 560 \text{ gal/min}$$

* (Ref 1): NOAA ATLAS 2, Precipitation-Frequency Atlas of the Western United States, Volume VII - Nevada

TOTAL CAPACITY OF PIT OVER 6 HR STORM

$$V_T = 5984 + 6 \times 60 \times 560 = 207,584 \text{ gal.}$$

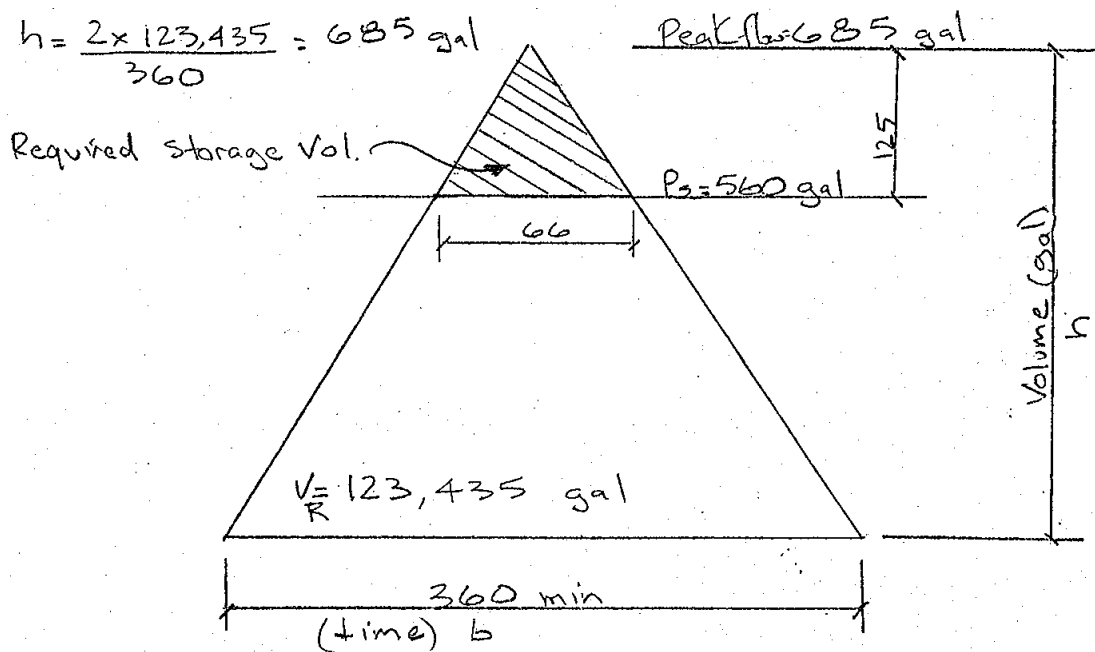
$$\underline{\underline{207,584 > 123,435 \therefore \text{OK}}}$$

CHECK PIT FOR PEAK FLOW

ASSUME TRIANGULAR HYDROGRAPH to Determine Flow coming into pit

$$V_R = \frac{1}{2}bh$$

$$h = \frac{2 \times 123,435}{360} = 685 \text{ gal}$$



$$\text{Req. Storage} = \frac{1}{2} \times 125 \times 66 = 4105 \text{ gal.} < 5984 \text{ gal.} \therefore \text{OK}$$

USE 20' x 20' x 6' DP PIT

Percolation PIT CALCULATION (AREA "D")

RAINFALL RUNOFF

$$\text{Total Area} = 85,275 \text{ ft}^2$$

$$\text{Landscaped Area} = 4,264 \text{ ft}^2$$

$$\text{Impervious Area} = 81,022 \text{ ft}^2$$

$$\text{Avg. Runoff coef.} = 0.87$$

Total rainfall precipitation for 10 yr, 6 hr. storm

$$P = 1.2" \text{ (ref 1)*}$$

$$\text{Total Runoff Volume} = \frac{1.2}{12} \times 85,275 \times 0.87 = 7420 \text{ CF} \\ = \boxed{55,495 \text{ gal.}}$$

Perc. Pit Design

Storage area: Use $\frac{1}{3}$ void ratio of the pit

$$\text{Percolation rate} = 1.0 \text{ gal/min/SF}$$

(per field test, see Appendix C)

Assume bottom surface area and $\frac{1}{3}$ side wall area to determine pit size.

Try 40 x 5 x 5' Deep pit

$$\text{Actual Storage Vol of pit} = \frac{40 \times 5 \times 5}{3} = \boxed{333 \text{ CF} = 2493 \text{ gal}}$$

Rate of percolation into soil

$$P_s = 40 \times 5 + \frac{1}{3} (40 + 40 + 5 + 5) \times 5 = 350 \text{ gal/min}$$

TOTAL CAPACITY OF PIT OVER 6 HR STORM

$$V_T = 2493 + 6 \times 60 \times 350 = 128,493 \text{ gal}$$

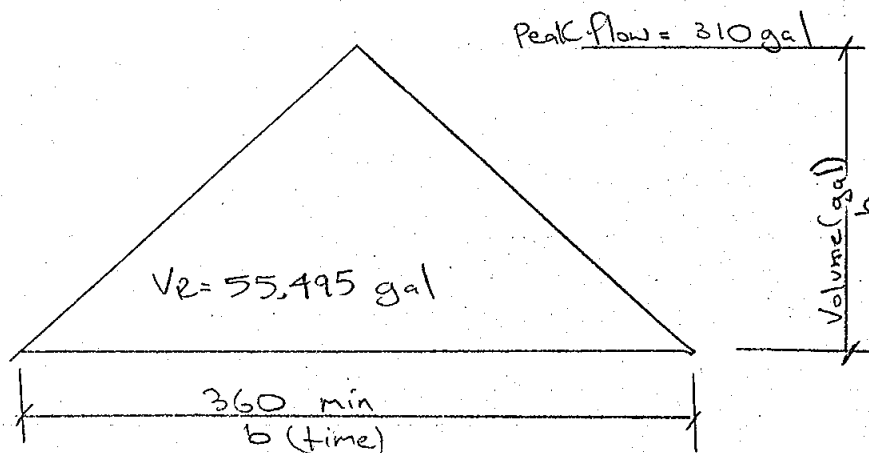
$$\underline{\underline{128,493 > 55,495 \therefore \text{OK}}}$$

CHECK PIT FOR PEAK FLOW

Assume triangular hydrograph to determine flow coming into pit.

$$V_R = \frac{1}{2}bh$$

$$h = \frac{2 \times 55,495}{360} = 310 \text{ gal}$$



Since peak flow req'd is less than percolation rate than no storage area is required.

$$310 < 350 \therefore \text{OK}$$

USE 40' x 5' x 5' Deep Pit

APPENDIX C

TEST PIT RESULTS



KLEINFELDER

December 1, 1987

File: 30-1328-06

Omni-Means
6121 Lakeside Drive, Suite 100
Reno, Nevada 89511

Attention Mr. Sean Crom

**SUBJECT: Field Infiltration Test for Dry Well Sizing
Washoe Medical Center Parking Lot
Reno, Nevada**

Gentlemen:

We have completed our field infiltration test at the proposed dry well location in Washoe Medical Center's southeast parking lot. We understand that surface runoff from the surrounding parking area is to be directed into the proposed dry well for percolation into the underlying native soil. This letter summarizes the soil conditions encountered, results of our field infiltration test, and a recommended design rate for stormwater infiltration.

Site Conditions

The site currently consists of a gravel parking area at the southwest corner of Lewis Street and Maine Street in Reno, Nevada. Dimensions of the lot are approximately 170 feet by 200 feet, and the surface is relatively flat.

Subsurface Conditions

A single test pit was excavated with a Case 580C backhoe to a depth of 8 feet. In the upper 4 to 4-1/2 feet we encountered a medium dense silty sandy gravel. This was underlain by a loose to medium dense slightly silty, fine to medium grained sand. Caving of trench sidewalls was noted below 4-1/2 feet.

The gravel and sand were in a slightly moist condition. No free ground water was encountered within the depths of exploration.

If, during construction of the dry well, subsurface conditions are encountered which vary from those described in this letter, we should be notified immediately, so that review and revision of our recommendations can be performed.

Field Infiltration Test

Our field test excavation was approximately 9 feet in length. Trench width was 3 feet in the upper 4-1/2 feet of trench depth, and increased to approximately a 5 foot width within the caving soils below depths of 4-1/2 feet. The trench orientation was east-west with the east end approximately 76 feet west of Maine Street (edge of pavement). The north edge of the pit was approximately 63 feet south of the back face of curb for Lewis Street.

The bottom of the trench was cleaned of loose material and cut as level as possible with the backhoe. No hand work was performed in the trench due to caving conditions. A 2 inch diameter slotted PVC pipe was placed in the center of the pit and a Class "C" backfill was placed to within 2 feet of the top of excavation.

Water for the test was added by a firehose connected to a nearby hydrant on Lewis Street. The quantity of water added was monitored with a Westpac water meter mounted on the hydrant outlet. Meter resolution was to the nearest gallon.

Prewetting was accomplished by adding 535 gallons of water over a period of 8 minutes. All of this water was dissipated in 3 to 3-1/2 minutes after shutting off the incoming water. Immediately afterward, a total of 2544 gallons was added over a period of 27-1/2 minutes, and a falling head test was performed.

Recommended Design Infiltration Rate

We understand that the bottom area and one-third of the sidewall area will be used in sizing the infiltration surface area for the dry well. Our field test showed an average infiltration rate of one gallon per minute per square foot of trench area for conditions approximating these design assumptions. In the infiltration test trench, a total contact surface area of 120 square feet has been provided. This capacity can be included in the total design of the system, provided that the prototype trench is pro-

December 1, 1987

File: 30-1328-06

Page: 3

perly connected to the final dry well construction by means of a deep gravel filled trench.

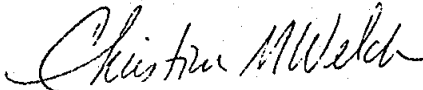
If you should have any questions regarding the contents of this report, please contact our Reno office.

Very truly yours,

KLEINFELDER



John Andreae
Staff Engineer



Christine M. Welch, C.E.
Project Engineer

JA:CMW:kns

Harding Lawson Associates

RECEIVED

DEC 14 1987

OMNI-MEANS, LTD.



December 11, 1987

3559,004.05

Washoe Medical Center
77 Pringle Way
Reno, Nevada 89520

Attention: Mr. Jim Lawing

Gentlemen:

PERCOLATION TEST RESULTS
DRAINAGE IMPROVEMENT STUDY
WASHOE MEDICAL CENTER
RENO, NEVADA

This letter presents the results of our percolation testing at the subject project. The work was requested by Mr. Sean Crom with OMNI MEANS LTD. as part of their drainage improvement study for the medical facility.

One percolation test pit was excavated using backhoe equipment at the location shown on Plate 1. The location of the test area was directed by Mr. Crom. The excavation measured approximately 12 feet in length (at the top), 2-1/2 feet in width, and 8-1/2 feet in depth. The general shape of the test pit and log of the materials encountered are shown on Plate 2. A 3 inch inside diameter (ID) perforated PVC drain pipe was installed in the center of the excavation and the pit back-filled to the original ground elevation, with clean 1-1/2 inch maximum size drain rock.

Both constant head and falling head tests were performed in the pit. Prior to performing the tests, approximately 10,000 gallons of water was pumped into the excavation through the 3 inch ID drain pipe. Water was obtained from a fire hydrant and hose. A water meter was attached to the hydrant to monitor the flow. The constant head test was performed by recording the amount of water with time while maintaining a constant water level. The falling head test recorded the drop in water elevation with time. The results of both the constant and falling head tests are presented on Sheet 1.

Engineers
Geologists &
Geophysicists

940 Matley Ln.
Reno
Nevada 89502

Telephone
702/329-6123

Alaska
California

Hawaii
Nevada

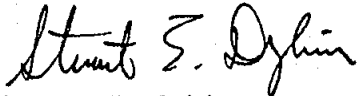
Texas

December 11, 1987
3559,004.05
Mr. Jim Lawing
Washoe Medical Center
Page 2

We trust this provides the information you require. If you have questions, please call.

Very truly yours,

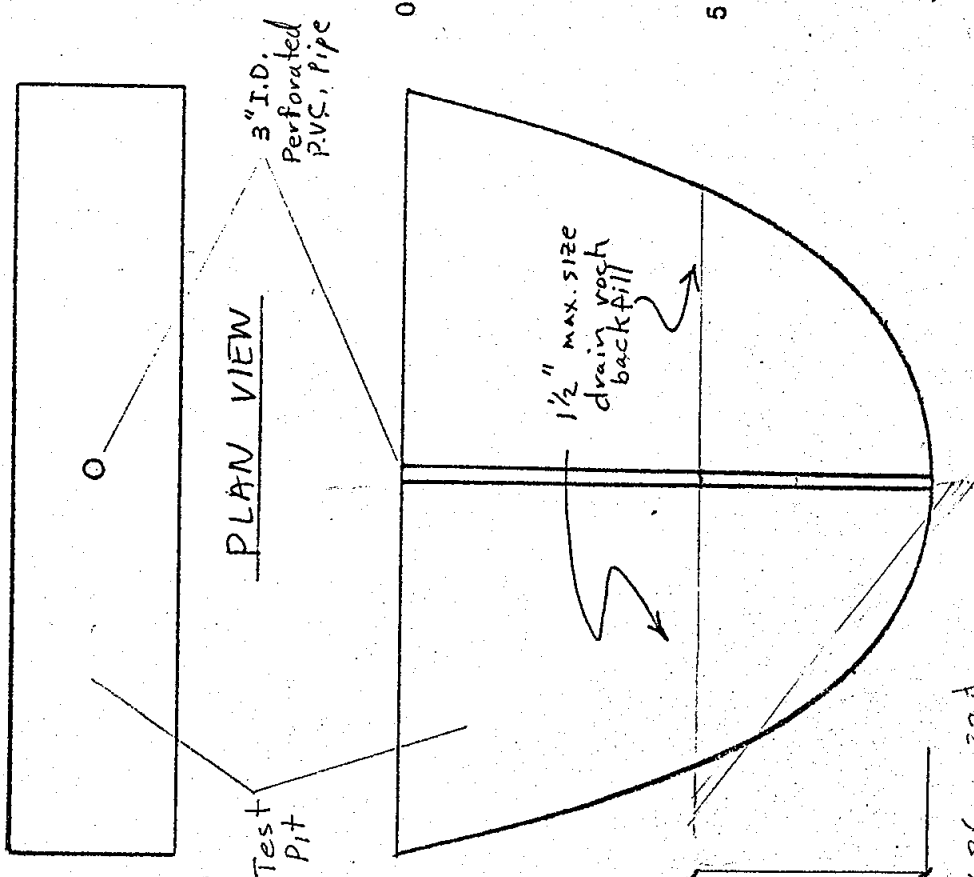
HARDING LAWSON ASSOCIATES



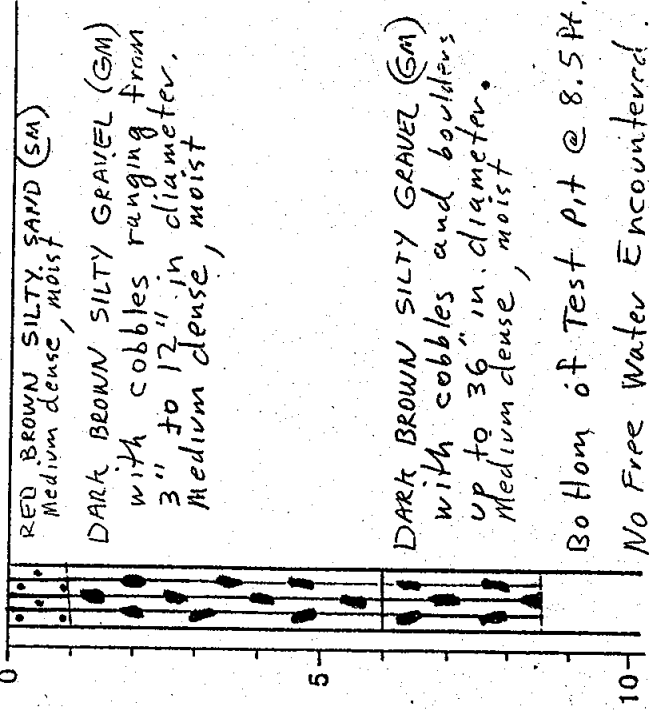
Stuart E. Dykins
Civil Engineer - 6665 (NV)

Enclosures

cc: OMNI MEANS LTD.
Attn: Mr. Sean Crom



LOG OF TEST PIT TPI
 Location See Plate 1
 Elevation — Date 12/4/87



3559/04
 12/10/87

SECTION VIEW
 scale 1" = 3ft.

10.8 x 2.5 = 27.2
 $3/4 \times 8.5 \times \frac{1}{2} \times 2 = 8.5$
 3559/04



Harding Lawson Associates
 Engineers, Geologists
 & Geophysicists

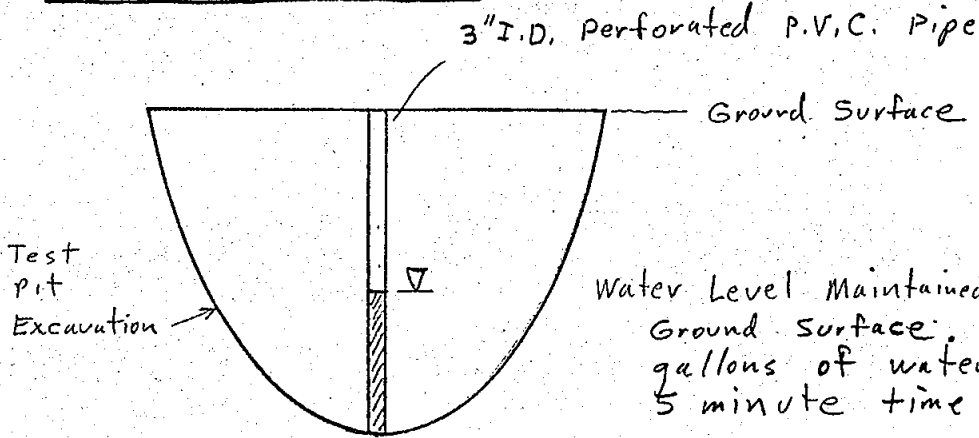
**LOG OF TEST PIT
 WASHOE MEDICAL CENTER
 RENO, NEVADA**

PLATE
2



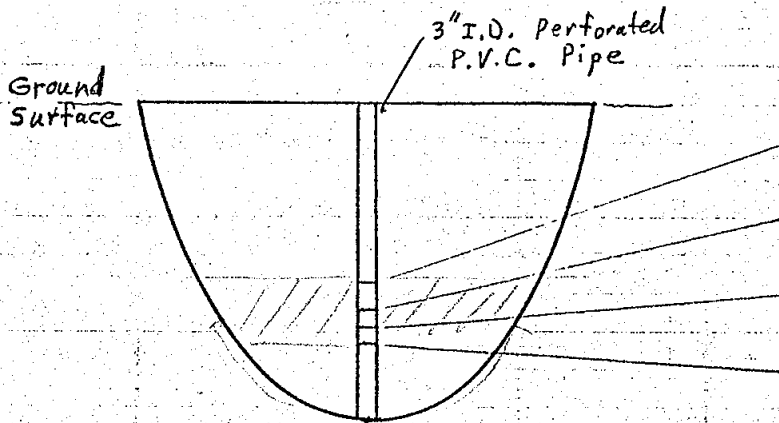
PROJECT Washoe Medical Center
 SUBJECT Infiltration Test

I. CONSTANT HEAD TEST



Water Level Maintained @ 4'-10" below Ground Surface. A total of 145 gallons of water added over a 5 minute time interval.

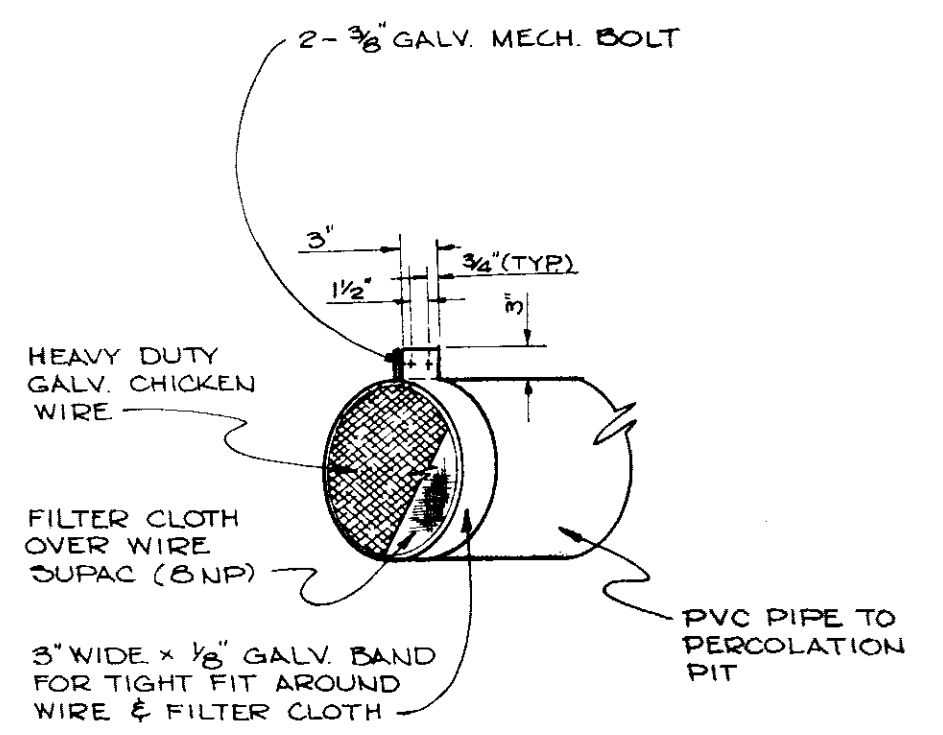
II. FALLING HEAD TEST



DEPTH of WATER BELOW G.S.	CUMULATIVE TIME min.	REMARKS
4'-10"	0	Start Test
5'-5"	5	
5'-11"	10	
6'-4"	15	End Test

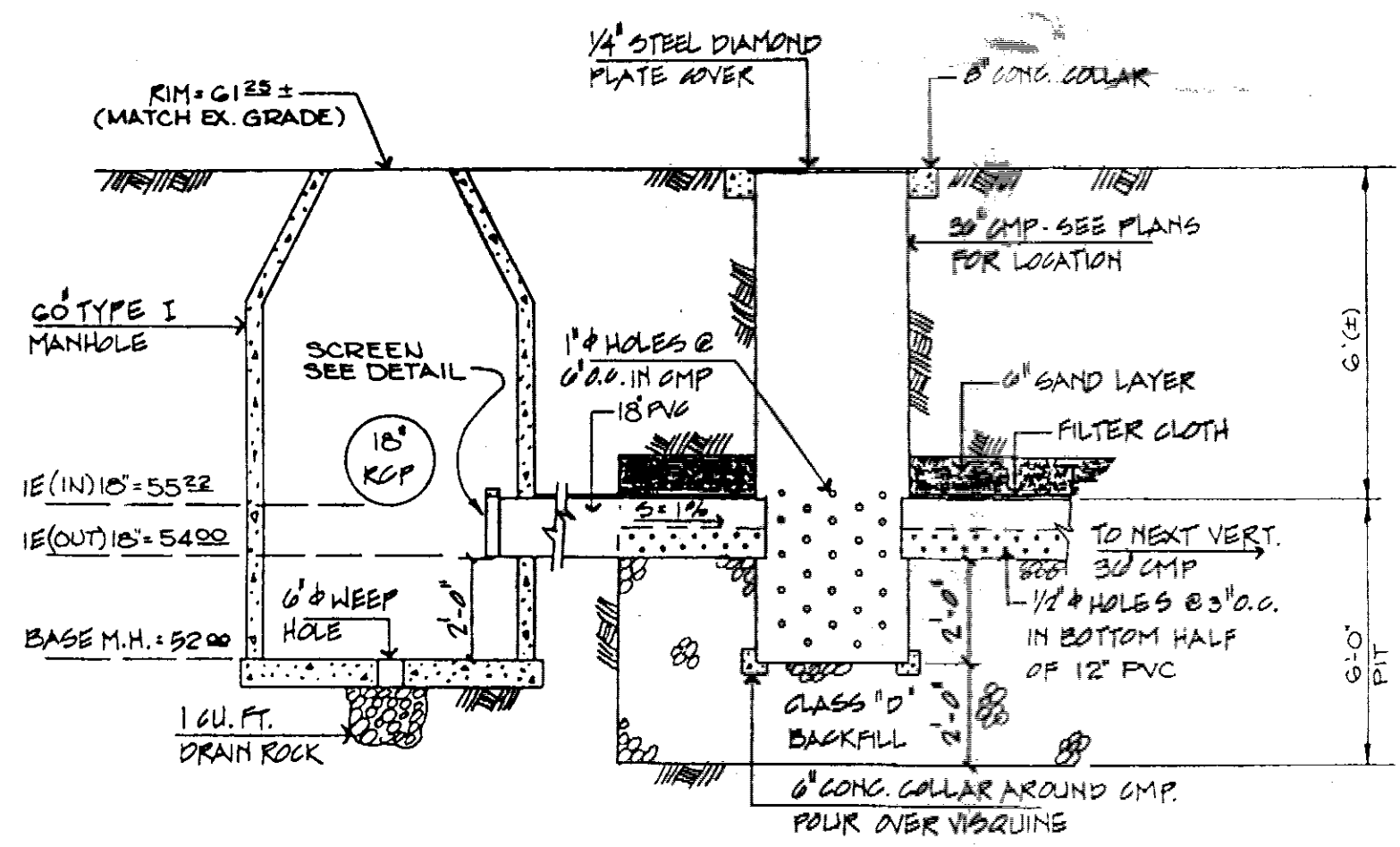
5' 16
 4' 10
 1' 6"

REVISIONS	BY



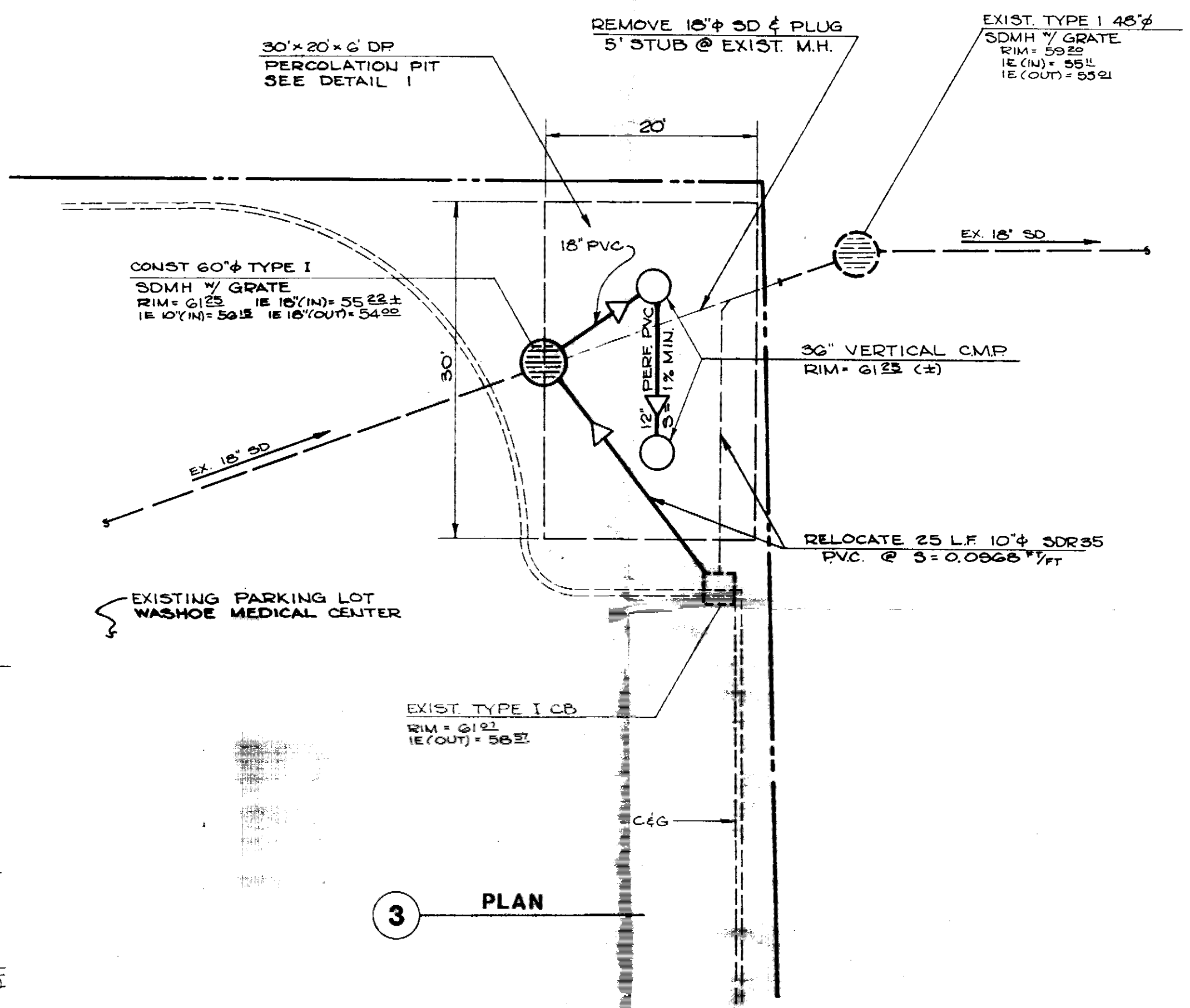
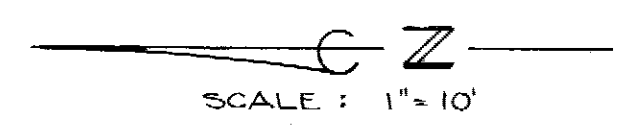
NOTE:
 1. OVERLAP WIRE & FILTER CLOTH A MIN. OF 3" ONTO PVC PIPE & SECURE W/ BAND.

1 SCREEN DETAIL
 N.T.S.



NOTES:
 1. SEE PLANS FOR PIT DIMENSIONS.
 2. STEEL PLATE OVER C.M.P. SHALL BE PAINTED BROWN AND SHALL HAVE 2 - 1/4" FINGER HOLES FOR REMOVAL.

2 PERCOLATION PIT DETAIL
 N.T.S.



3 PLAN

OMNI-MEANS, Ltd.
Engineers & Planners
 6121 Lakeside Drive • Reno, Nevada 89509
 7509 Madison Avenue • Suite 120
 Citrus Heights, California 95610

PERCOLATION PIT
WASHOE MEDICAL CENTER
REVISED DRAINAGE PLAN
 77 PRINGLE WAY
 RENO, NEVADA

Date DEC. 1987
 Scale AS NOTED
 Drawn R.E./JOAN
 Job 1758-01
 Sheet

